AD 386

Clarification of notch dimensions and shear resistances in SCI P358 (Green Book on Simple Joints)

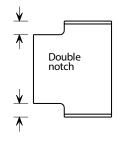
On page T-4 of SCI P358⁽¹⁾, Table G.1 Note 4 states that for double notched beams, the remaining depth of web is taken as the end plate length. This is misleading and provides no information on single notched beams. This AD explains what notch dimensions were assumed and how the quoted shear resistances were calculated for single and double notched beams.

Single notch

Greater of 50 mm and clearance *n*

Greater of 50 mm and clearance n For 406 \times 178 and larger

25 mm For 406×140 and smaller



Notch dimensions

For single notched beams, the notch depth was taken as the larger of 50 mm or the clearance *n* as given in SCI P363^[2] (Blue Book). For most beams therefore, the notch aligns with the top of the end plate, set 50 mm below the top of the beam. For large beams, where the thickness of the flange plus root is greater than 50 mm, it is assumed that the end plate is lowered to clear the root, and the notch depth is correspondingly increased.

For doubly notched beams, the upper notch follows the rules given above for single notches. The lower notch similarly follows these rules for large and medium sized beams. For 406×140 and smaller, the lower notch depth is simply taken as 25 mm. The rules are given in the figure above.

Calculation of shear resistance

When compiling the resistance tables (Tables G.4 & G.5), the first step was to determine the maximum notch length which could accommodate the shear resistance quoted for the beam without a notch. In many cases, the

Notch dimensions in the Green Book

Greater of 50 mm

and clearance n

maximum notch length was zero, or some small dimension which had no practical benefit. In these cases, a reasonable notch length was set as 100 mm and the resistance back-calculated (using an iterative process) for this geometry. In this process, the applied shear was increased until the applied moment at the notch equalled the moment resistance. An iterative process was required as the moment resistance is reduced in the presence of high shear; the reduction varies with the applied shear.

In the resistance tables, if the maximum length is quoted as 100 mm, it will be associated with a reduced shear resistance, indicating that the process above has been followed. For lengths longer than 100 mm, the resistance will be that for an un-notched beam. Occasionally for double notched beams, where 'N/A' is shown in the shear resistance column, it indicates that after the notches have been removed (following

the guidance given above) the remaining depth of web is less than the depth of the end plate. In these cases the resistance of a non-standard connection will have to be determined by calculation.

In many cases, the dimensions of the supporting beam may dictate the size of the notch. In these cases the resistance will have to be determined by calculation.

References:

- [1] SCI P358 Joints in Steel Construction: Simple Joints to Eurocode 3. (2014)
- [2] SCI P363 Steel Building Design: Design Data. (Updated 2013).

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Eurocode verification of a runway beam subject to wheel loads – Part 1

 $\gamma_{\rm M,ser}$ is to be taken as 1.1, according to the UK National Annex.

EC3-6 does not indicate where precisely stresses should be checked at positions 0, 1 and 2. At the extreme fibres on the underside of the flange, the global bending stress $\sigma_{\rm x,Ed,ser}$ is at a maximum, as is the local transverse bending stress $\sigma_{\rm oy,Ed,ser}$, but the shear stress is zero. At other locations, the shear stress will be combined with a reduced global bending stress. It is conservative simply to combine maximum stresses, especially as the shear stress based on clause 6.2.6 of BS EN 1993-1-1 is likely to be small.

2.3 Vibration of the bottom flange

A further serviceability requirement concerning runway beams is the need to avoid noticeable lateral vibration of the bottom flange. Clause 7.6 of EC3 6 recommends that the slenderness ratio of the bottom flange L/i_z should be limited to 250, where i_z is the radius of gyration of the bottom flange and L is the distance between lateral restraints.

- 1. LTBeam software, available from www.cticm.com
- 2. SN003 Elastic critical moment for lateral torsional buckling, available from www.steel-ncci.co.uk